

Disposal Field Design Considerations & Details

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Permit Number: PRPSW	Roll Number:	

The following information is to accompany the Private Sewage Disposal Permit Application for a Disposal Field. **Required Information:** Private Sewage Disposal Permit Application - Completed (please put N/A in spaces which are not applicable). ☐ Signature of Applicant on Permit Applicant Declaration Disposal Field Design Considerations & Details - This form Completed. ☐ Site Evaluation Diagram - Appendix A - Attach a detailed site diagram including the system location in relation to buildings, distance to water supply and /or surface water bodies, and other pertinent information (as per Part 7 of the Private Sewaae Standard of Practice 2009). Soil Log Report from two (2) test pits with Soil Analysis Report - Appendix B - Completed. ☐ Worksheets - Complete both 'Primary Effluent Treatment Field' - Appendix C and 'Pressure Distribution, Orifice, Pipe & Pump Sizing' - Appendix D Specifications for System Components - Attached for Initial Treatment Component Design Details, including Septic Tank, Dose Tank, Effluent Pump. Detailed System Schematics and Drawings - Attached Any other qualifications of limitations that in your opinion as the designer/installer are needed. This private sewage system is for a _____ (# of) bedroom single family dwelling. Total peak wastewater flow per day used

property. The design reflected in the flowing applies, and meets the requirements of the current Alberta Private Sewage Systems Standards of Practice adopted under the Safety Codes Act to achieve effective treatment of the wastewater from this residence.

The sewage system includes a septic tank and disposal field. This system is suitable for the site and soil conditions of your

in this design is imperial gallons. The average operating flow is expected to be gallons per day.

Wastewater Characteristics:

Wastewater Peak Flow:

- The development served is a (# of) bedroom single family dwelling.
- Fixture Unit Loads (please check all that apply):
 - ☐ Main Bathroom = 6 fixture units
 - ☐ Ensuite with Shower = 6 fixture units

	☐ Kitchen Sink = 1.5 fixture units	
	☐ Laundry Stand Pipe = 1.5 fixture units	
	☐ Bathroom in Basement = 6 fixture un	
		identified in the review of this development
		Please include total volume used by these
		per Table 2.2.2.3 of Alberta Private Sewage
	Systems Standard of Practice 2009.	per rubic 2:2:2:3 of Alberta i rivate sewage
	_	
	No high-volume planibing fixtures we	ere identified in the review of this development
	(examples: garburator, soaker tubs).	
	• Total plumbing fixture unit load in this residence:	
	(based on review of the building)	
•	Based on the total plumbing fixtures, Imp. 0	Gallons per day is required to be added to the
	base peak daily flow.	
•	• Note: When the combined total of fixture units ex	ceeds 20 in an occupancy unit, add 50L for each
	fixture unit over 20 (as per Table 2.2.2.2.A. of the	
	Practice 2009)	
	•	
Summary:		
•	Total peak daily flow used in the design is:	Imp. Gal/day
	Base Flow:	Imp. Gal
	Additional Flow:	Imp. Gal
	Additional Flow.	p. Gai
Wastewater Strength:		
wastewater strength.		
	Characteristics of the development were consider	red to assess sewage strength. No garbage
	grinders or other characteristics were identified t	
	be exceeded.	,
	The Required wastewater strength for the design	ic·
		15.
	• BOD 220 mg/L	
	 TSS 220 mg/L 	
	 Oil and Grease 50mg/L 	
Wastewater Flow Variati	an Cancidarations	
wastewater riow variati	on considerations.	
	The characteristics of this development indicate w	vastewater flow volumes will not vary
	substantially during the day, or from day to day.	
	needed.	as a result, no now variation management is
Site Evaluation Findings:		
Site Evaluation Diagram:		00/110
•	• Lot area:	ac / Ha
_		
Ц	The dimensions of the property are shown in the	Site Evaluation Diagram, attached in Appendix
	A .	

	The site evaluation assessed the area within a 100m (330ft system design. The design conforms to all distances set ou including the distances to adjacent property features. No snoted.	t in the Standard of Practice (SOP),
	Pertinent features identified during the site review and the identified on the Site Evaluation Diagram - Appendix A .	e required setback distances are
Soil Evaluation:		
	Two (2) soil excavations have been investigated on this sit	
	Test Pit 1 is located at the proposed location of the disposa	
	☐ Soil Logs - Appendix B have been completed for Test Pit 1	and Test Pit 2.
	The location(s) of the Test Pits are shown on the Site Evalu	ation Diagram, <i>Appendix A.</i>
Restrictive Soil Cond		
	 Key aspects of the soil conditions that must be addressed i 	•
	• A restrictive layer exists at feet below	w the surface as indicated by:
	therefore, a disposal mound is required.	
Limiting Soil Condition	ion - Effluent Loading Rate Selection: • The key soil Characteristics of the design is based on (soil t	ype):
	This soil type has a effluent loading rate of:	
	The effluent loading rate for secondary treated effluent on Imperial Gallons/day/ft ²	this soil is :
Effluent Linear Load	ling Rates and Design Considerations:	
	 There is a shallow restrictive soil layer at this site. The efflusion soil so linear loading rates must be applied: The dominant soil characteristic is: Infiltration distance to the restrictive layer: 	uent must move laterally through the
	• Linear Loading Rate:	Imperial Gallons/day/ft ²
	☐ System Component Designs have been attached.	
Initial Treatment Co	omponent Design Details:	
	☐ Details of the initial treatment components are required for	or this design are attached.

Septic Tank:	
	• The working capacity of the septic tank specified for this design is Imp. Gal.
	Tank Model Number:
	☐ Specifications for the Model of Septic Tank used are attached.
	The minimum working capacity required for this development is Imp. Gallons based on Table 4.2.2.2 of SOP 2009 for a bedroom house (Imp. Gal/day plus the additional flow of Imp. Gal, as summarized above under Wastewater Characteristics).
	•
	 Burial depth of the septic tank at finished grading will be inches above the top of the tank. This tank is rated for a maximum burial depth of : Insulation of the tank required?
	☐ Yes -
	□ No
D T 1	
Dose Tank:	
	The dose chamber is integral to the septic tank. It has a total capacity of Imp. Gal. This is sufficient capacity to deliver the Imp. Gal required for each dose of effluent. It also provides Imp. Gal emergency storage above the high effluent alarm setting (a minimum of one (1) day emergency supply is required). □ Specifications for the Dose Tank are attached.
High Liquid Level A	slarm:
	 Alarm Model Number: The alarm is set to activate inches above the floor of the dose tank.
Effluent Filter:	
	Filter Model Number:
	• Filter diameter: inches
	The Filter creates a head loss of 0.5 feet at its rated flow of 80 Imp. Gal/min. A 5.5 foot pressure head allowance has been included in the pump selection to allow for partially clogged conditions.
	A one year service interval is expected with typical flow volumes and wastewater characteristics.
Soil Treatment Co	mponent Design Details:
	☐ The system designed for this site is a septic tank and treatment mound.
	Key design requirements:Expected Peak Daily Flow:
	• Soil Loading Rate:
	• Linear Loading Rate:
	Minimum in-site soil infiltration area:
	• Soil Infiltration Surface Area:
	• Minimum Soil Infiltration Width:

	The location of the disposal field and the layout of laterals are shown on the Site Evaluation Diagram, Appendix A
	The trench bottom sizing worksheets are completed and attached - <i>Appendix C</i> .
	The layout of the laterals have been included with the detailed system schematics and drawings .
Effluent Distribution Des	ign Detail:
Effluent Pressure Distribu	
	The Pressure Distribution, Orifice, Pipe & Pump Sizing worksheets are completed and attached - Ap The Effluent Pressure Distribution is:
Effluent Pressure Distribu	tion Lateral Design:
•	The foot long pressure distribution laterals are center fed resulting in (# of) pressure distribution laterals. Each lateral is inch schedule 40 PVC pipe. Each lateral has (# of) 1/8 inch orifices drilled at foot spacing. Orifices will be offset between the two laterals along its length.
•	The design achieves a minimum of 5 foot pressure head at each orifice, resulting in a design flow of Imp. Gal/Minute from each 1/8 inch orifice.
	There are (# of) orifices throughout the effluent pressure distribution system resulting in a total flow of Imp. Gal/minute. An additional 3.2 Imp. Gal/minute is added for the 1/4 inch drain back orifice drilled at the lowest elevation of the effluent piping in the dose tank to achieve drain back of the laterals and supply piping.
•	Total flow required for the effluent pressure distribution system is Imp. Gal./minute.
Pressure Head: Pressure Head Requireme	ents:
·	The total length of supply piping from the pump to the start of the pressure distribution laterals is feet. The supply piping is 2 inch Schedule 40 PVC pipe . The allowance for equivalent length of pipe due to fittings is feet of pipe. The equivalent length of pipe is feet. This is detailed in <i>Appendix D</i> .
Pressure Head Loss Due t	o Friction:
•	The friction loss through the feet of piping and filter at the flow of Imp. Gal/min is feet of head pressure.
	derations: Allowance for head loss through the effluent filter under partial plugging is 5.5 feet. Allowance for pressure head loss along the pressure distribution laterals is 1 foot.

	The total pressure head required to overcome friction loss is feet pressure head.
Pressure Head to Meet	Vertical Lift Requirements Included: ☐ A pressure head at each orifice of 5 feet.
	·
	 Light distance of effluent from the low effluent level in the tank to the pressure distribution laterals is feet.
	rect.
	• The vertical lift and friction loss results in a total pressure head requirement of
Downer Conneiling tions	
Pump Specifications:	• Demand for this assessment distribution between twenty in the Callerinate
	 Demand for this pressure distribution lateral system is Imp. Gal/minute at feet of pressure head.
	The pump capacity exceeds these demands to allow for variations in the design and decreased
	pump performance over time.
	Effluent Pump Model specified for this system:
1	• Horsepower of Effluent Pump: The pump specifications with the effluent distribution system demands plotted on the pump curve
	are attached.
Effluent Dosing Volume	e and Control Settings:
	• The volume of effluent discharge in a single dose event needs to be less than 20% of the daily flow,
	which is gallons.
	• The volume of an individual dose must be at least 5 times the volume of the pressure distribution
	laterals, which is Imp. Gallons.
	• Therefore, the individual dose volume selected is gallons.
Effluent Level Float Cor	ntrol Settings:
	• The volume in the feet of 2 inch PVC effluent supply is gallons.
	•
	The total individual dose volume determining float settings is Imp. gallons to fill the
	effluent supply line and deliver the Imp. gallons per dose.
	• The dose tank dimensions result in gallons per inch of depth.
	• The float control elevations shall be set at:
	• inches between float off and on elevations.
	Off: inches off floor of dose tank.
	On: inches off floor of dose tank. off floor board on playetings get out in this desire.
	 off floor based on elevations set out in this design.
1	

The effluent level control floats will be attached to an independent PVC pipe float mast.

Operation Monitoring:
The following components are included in the system design and detailed drawings showing locations have been attached: Monitoring Ports - provided at both ends of the laterals to enable inspection of the effluent ponding depth that may result. Pressure Distribution Lateral Clean Outs - provided at the end of each pressure distribution lateral with access to grade through an access box suitable for its purpose and anticipated traffic.
Effluent Quality Sampling:
☐ Effluent samples can be taken from the effluent dose tank if required.
In the following activities need to be conducted to commission the system: Clean the septic tank and effluent chamber of any construction debris. Flush effluent distribution laterals. Conduct a squirt test to assess the residual head pressure required by the design is achieved and that the volume from each orifice is within allowed tolerances. Confirm the correct float levels and ensure this delivers the dose volume required by this design. Department of the installed system will be provided to the owner in accordance with Article 2.1.2.8. of the Standard of Practice.
Signature and Closing by the Designer/Installer:
This design has been developed by This design meets the requirements on the Alberta Private Sewage System Standard of Practice 2009 unless specifically noted otherwise and in such case special approval is to be obtained prior to proceeding with installation of this design.
Signature of Designer/Installer:



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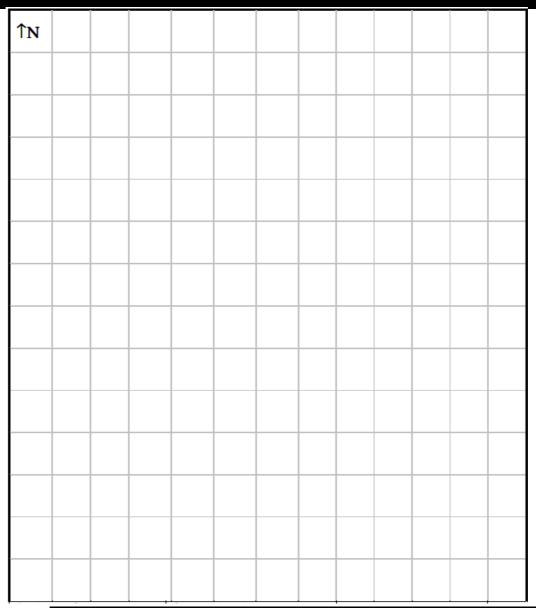
Appendix A - Site Evaluation Diagram:

Please show the proposed location of the onsite sewage system and indicate the distance	s from	the following
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Bedrock Trees Driveways Easement Lines

OutcropsFloodplainsExisting Sewage SystemsDitches or InterceptorsBuildingsWellsUnderground UtilitiesBanks or Steep Slopes

Property Lines Waste Sources Soil Test Pits Fills



Please indicate:

Drainage Course	Slope Direction	Test Pit 1	Test Pit 2
~		Test Pit 1 🗖	Test Pit 2 🗖



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Appendix B - Soil Log Report: Test Pit 1

Owner Name or Job ID.	or Job ID.			Legal Lan	Legal Land Location					Test Pit G	Test Pit GPS Coordinates	
LSD-1/4	Sec	Twp	Re	Mer	Lot	Block		Plan	<u> </u>	Easting	Northing	ing
	3		e e		3			1				f
Vegetation notes:	SS:					Ove	Overall site slope % Slope position of test pit:	ıt pit:				
Test hole No.		Soil Subgroup			Parent Material		Drainage		Depth of Lab sample #1	mple #1	Depth of Lab sample #2	ple #2
							,					
Hori- zon (c	Depth (cm) (in)	Texture	Lab or HT	<u> </u>	Colour	Gleying	Mottling	Structure	Grade	Consistence	Moisture	% Coarse Fragments
Depth to Groundwater	rater	igg			Restr	Restricting Soil Layer Characteristic	haracteristic					
Depth to Seasonally Saturated Soil	ly Saturated S	oil			Deptl	Depth to restrictive Soil Layer	Layer					
Site Topography					Deptl Desig	Depth to Highly Permeable Layer Limiting Design	ble Layer Limitin	PÖ.				
Key Soil Characteristics applied to system design effluent loading	eristics applie luent loading	ed to			-							
Weather Condition notes:	n notes:	_										
Comments: such as root depth and abundance or other pertinent observations:	ıs root depth aı	nd abundance	or other p	ertinent o	bservations:							



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Appendix B - Soil Log Report: Test Pit 2

Alberta Private Sewage Treatment System Soil Profile Log Form	Name or Job ID.	Land Location	1/4 Sec Twp Rg Mer Lot Block Plan Easting Northing	on notes: Overall site slope %	Slope position of test pit:	No. Soil Subgroup Parent Material Drainage Depth of Lab sample #1 Depth of Lab sample #2	Depth Texture Lab or Colour Gleying Mortling Structure Grade Consistence Moisture % Coarse (cm) (in)			roundwater Restricting Soil Layer Characteristic	asonally Saturated Soil Depth to restrictive Soil Layer	Design Design	Key Soil Characteristics applied to system design effluent loading	andition notes:	Comments: such as root depth and abundance or other pertinent observations:
Alberta Private Sev	Owner Name or Job ID		LSD-1/4 S	Vegetation notes:	,	Test hole No.	Hori- Depth zon (cm) (in)			Depth to Groundwater	Depth to Seasonally Saturated Soil	Site Topography	Key Soil Characteristics applic system design effluent loading	Weather Condition notes:	Comments: such as root de



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Appendix C - Primary Effluent Treatment Field

Trench Bottom Surface Area & Length Sizing

This design worksheet was developed by Alberta Municipal Affairs and Alberta Onsite Wastewater Management Association.

The complete system is to comply with Alberta Private Sewage Standard of Practice 2009

This worksheet does NOT consider all of the requirements of the Mandatory Standard.

Use only Imperial units of measurement throughout (feet, inches, Imperial gallons, etc...)

Step 1) Determine the expected volume of sewage per day:											
Note: Use Table 2.2.2.2.A. (p.30) & 2.2.2.2.B. (p.31) as a guide to determine expected volume of sewage per day. Provide allowance for additional flow factors as detailed in Table 2.2.2.3. (p.32) Expected Volume of Sewage per Day											
Day	$\overline{}$										
Assure that the sewage strength does not exceed the requirements of 2.2.2.1.(2) (p 30)		F1									
Effluent quality must meet the requirement of Article 8.1.1.6(1)(a) page 97.	_										
Step 2) Determine the design soil effluent loading rate:											
Soil Effluent Loading F	Rate										
[From >30 - 150 mg/L co	lumn]										
&	Imp. gal/ sq.ft./day	F2									
Texture Structure Grade											
Note: Effluent loading rate MUST be determined from soil texture, structure, and grade classification according to Imperial Table A.1.E.1. (p.151).											
Note: Ensure infiltration loading rate chosen does not exceed loading rates as set out in 8.1.2.2. (p. 101).											
Step 3) Calculate the required infiltration surface area for the soil BEFORE area reduction factors:											
Expected Volume of Sewage Soil Effluent											
per day Loading Rate Soil Infiltration Are	a Required										
		F3									
Imp. gal/day Imp. gal/sq. ft/day sq.ft.	•										
From F1 From F2											
At no time shall primary treated effluent loading rates											
exceed the loading rates for secondary treated effluent.											
Step 4) Type and width of trench bottom used:											
Actual Pipe & Rock Trench Width in inches.											
	feet	F4									
inches ÷ 12 =	ieet	14									
Actual Chamber Width in inches											
	£4										
inches ÷ 12 =	feet	F4A									



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Appendix C - Primary Effluent Treatment Field

Step 5) Calculate optional credited loading rates for distribution systems:										
Primary treat	ed effluent	is required	I to have a	minimum 5	feet Vertical Separat	tion below in	nfiltration 2	one.		
		_			_		c		dth of Trendeet)	ch
Pipe & Rock T	Pipe & Rock Trench			dth in Feet m F4	x	1	=	3 ft. 1	Maximum	F5
Pipe & Rock T Distribution	Pipe & Rock Trench with Pressure Distribution			dth in Feet m F4	x	1.2	=	3 ft. 1	Maximum	F5
Chambers - (Chambers - Gravity Feed			amber in Feet 1 F4A	x	1.1	=	3 ft. l	Maximum	F5A
Chambers - F Distribution	Pressure			amber in Feet n F4A	x	1.3	=	3 ft. li	Maximum	F5B
Step 6) Determine linear feet of trench required:										
	Infiltration Area Required sq. ft. From F3 or F3A Credited Width of Trench ft. From F5, F5A or F5B Credited Width of Trench ft. From F5, F5A or F5B					F6				
Total Trend	ep 7) Select number of weeping lateral trenches and Dete Total Trench Length Required Number of in Field Late						Leng	th of Each V Lateral Trer		
From	F6	ft.	÷	Designe	F7 er Determination	=		1	feet	F8
It is good pra	It is good practice is to make the overall field area long and narrow when possible on a particular site.									
Step 8) Summary	Step 8) Summary:									
F1					Imp. gal/day - Dail	y Flow, inclu	uding any	additional fi	ixtures.	
F2					Imp. gal/sq.ft Effluent Loading Rate.					
F3 or F3A	or F3A				Sq. Ft Soil Infiltration Surface Area.					
F4					Ft Actual width	of Gravel T	rench or 0	hamber.		
F5A or B	A or B				Ft Credited Width of Gravel Trench or Chambers.					
F6					Ft Total trench	length requ	ired.			
F7				Number of weeping lateral trenches.						
F8					Ft Length of ea	ch weeping	lateral tre	nch.		



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Appendix D - Pressure Distribution, Orifice, Pipe & Pump Sizing

This design worksheet was developed by Alberta Municipal Affairs and Alberta Onsite Wastewater Management Association.

The completed installation is to comply with Alberta Private Sewage Standard of Practice 2009.

This worksheet is for use in Alberta to: size the orifices in distribution lateral pipes, size effluent delivery piping, and to calculate the required capacity and pressure head capability of the effluent pump.

It can be used for: calculating deliver of effluent to laterals in disposal fields, mounds and sand filters.

This worksheet does NOT consider all of the mandatory requirements of the Standard.

It is intended for use by persons having training in the private sewage discipline.

Note: Page numbers refer to the Private Sewage Systems Standard of Practice 2009.

Use only Imperial units of measurement throughout (feet, inches, Imperial gallons, etc...).

Step 1) Select the pressure head to be maintained at the orifices: Minimum pressure at the orifice: 3/16" or less orifice = 5 ft. Minimum - 2.6.2.5 (1), (p 48) larger than 3/16" orifice = 2 ft. Minimum - 2.6.2.5 (1) (p 48) Design pressure at lateral orifices Note: worksheet will not provide an adequate design if laterals are at different elevations. Differing elevations will result in a different pressure head and volume of discharge at the orifices in each lateral. Additional considerations must be made for laterals at differing elevations.	P1
Step 2) Select the size of orifice in the laterals: Minimum size: 2.6.1.5. (1)(e) p. 46 1/8" Orifice Diameter selected in. Note: larger sizes are less likely to plug.	P2
Step. 3) Select the spacing of orifices and determine the number of orifices to be installed in distribution laterals: Length of Distribution Lateral From system design drawings Spacing of Orifices selected for Resulting number of orifices per lateral ft. ft. Select a spacing of orifices to attain even distribution over the treatment area: Maximum spacings are determined for: * 5 ft. Primary treated effluent: 2.6.1.5 (e) (pp. 46 - 47) * 3 ft. Secondary treated effluent: 8.1.1.8 & 2.6.2.2 (c) (pp 98 & 47 - 48) * 3 ft. On sandy textured soils: 8.1.1.8 (p. 98) X From P3a Number of Laterals If laterals are of differing lengths, calculate each separately and add the number of orifices together.	P3a P3b
Step 4) Determine the minumum pipe size of the distribution laterals: Enter the system design information into the 3 boxes below. If distribution laterals are of differing lengths, each lateral must considered separately. Orifice Diameter Length of Distribution Lateral in. From P2 From System Design Drawings From P3a Use Table A.1.A. (pp 140 - 143) when applying the information entered in this step to determine the minimum size of the distribution lateral pipe. Size of Distribution Lateral Pipe From Table A.1.A.	st be



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Appendix D - Pressure Distribution, Orifice, Pipe & Pump Sizing

Step 5) Determine the total f Total Number of Orifices in all laterals From P3b	Gal/min for e at Head Press X From Tab (pp 144	Imp. gal /min.	Total flow from all orifices	lateral Imp. gal /min. P5
Step 6) Select the type and s Use Tables A.1.C.1 to A.1.C.4 decision. A larger pipe will rec	(pp 146 - 149) to aid in duce pressure loss.	Type of pipe used for effluent delivery	Pipe size selected	inch P6
pipe size selcted will affect the			-	eet per second. The
Step 7) Calculate the equiva Insert total from Worksheet Distribution Worksheet		Ec	ngs: quivalent Length of All Fittings For Pressure Loss	ft. P7
Step 8) Calculate the equivaloss: Length of Piping (ft) Length from pump to farthest end of distribution header supplying laterals.	Equivalent Le	ngth of Fittings Loft) = Use	ength of Pipe for Friction Loss (ft) sed to determine total pressure ead loss due to friction loss in ping.	rals for pressure
Total Length of Pipe for Friction Loss From P8 Don't forget to divide the length by 100 feet to match the factors in the tables.	Friction I 100 feet Divide by 100 ft.	of pipe ft. 1.C. On pp 146 - 150	Delivery Piping Pressure Head Loss	ft. P9



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Appendix D - Pressure Distribution, Orifice, Pipe & Pump Sizing

Step 10) Calculate the total	I pressure h	ead required at numr	n:	
Delivery piping pressure loss	i pressure ii		ft.	From P9
Lift distance of effluent from effluent level in tank to orifices		+	ft.	Measure from lowest effluent level in tank to elevation of orifices.
Design pressure at orifices		+	ft.	From P1
Head loss allowed if an inline filter is used in pressure piping		+	ft.	Explain Pressure Loss Allowed if Applied
Add 1 ft to allow for pressure loss along the distribution lateral		1	ft.	
Total minimum press				P10
lmp. gal/min requi supply orifice	red to		ft.	PIO
Step 11) Select the size of t total flow requirement for p		ck orifice if used and	determine t	he flow from the drain back orifice. Then calculate
Size of Drain Back Orifice	Determine Head Pre Drain Bac	ssure at late	w from all ral orifices	Total Imp. Gallons per Minute from the pump
in.		Imp. gal /min	Imp. ga /min	Imp. gal P11
	Use press from P10 to from Ex Table A	o find flow tended	rom P5	
Step 12) Details of the pum	np specifica	tions required:		
Required Flow Rate (Imp. gal/min)		Required Pressure Head (ft)	e	
	@			Select the appropriate pump by reviewing the pump curve of available pumps. Select a
From P11		From P10 Required Flow Rat (US gal/min)	te	pump that exceeds the requirments set out in this step by approximately 10% considering both pressure head and volume.
Imp. gal (<mark>P11</mark>) multiplie = U.S. gallons	-	(90)		
Sten 13) Consider the num	ning demar	ids of the system. If t	hev are con-	sidered excessive, redesign the pressure

Step 13) Consider the pumping demands of the system. If they are considered excessive, redesign the pressure distribution system and recalculate the pump demands.



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Appendix D - Pressure Distribution, Orifice, Pipe & Pump Sizing

Worksheet "Appendix A" Determine Equivalent Length of Pipe due to fittings in piping system.							
Determine the equivalent length of pipe to allow for friction loss due to fittings in the piping system:							
	Number of Fittings		Friction loss as per Table A.1.C.5 or 6 (p. 150)		Total		
90° Elbows		x		=			
45°Elbows		x		=	+		
Gate and Ball Valves		x		=	+		
Tee-on- Branch (TOB)		x		=	+		
Tee-on-Runs (TOR)		x		=	+		
Male Iron pipe Adaptors (M/F Threade	d Adaptors)	x		=	+		
					=		
Total Equivale	ent Length of pipe to allow ng system	for	(Enter this total, B	ox P7)			